QUAD CITIES INTERNATIONAL AIRPORT

METROPOLITAN AIRPORT AUTHORITY
OF ROCK ISLAND COUNTY, ILLINOIS

ADDENDUM NO. 6
Invitation for Bids
Kirk Hangar Foundation Installation

This Addendum No. 6 is hereby issued to modify and supplement the Invitation for Bids ("IFB") dated January 28, 2026, for the **Kirk Hangar Foundation Installation** project. All provisions of the IFB not expressly modified herein shall remain in full force and effect.

1. Addition of Door Rail Foundation Shop Drawings Exhibit

The IFB is hereby amended to include a **Door Rail Foundation Shop Drawings Exhibit**, identified as **Schedule E**.

Schedule E is provided for informational and coordination purposes and is intended to assist bidders. Bidders are responsible for verifying all site conditions and dimensions as required by the IFB.

2. Incorporation by Reference

Schedule E is hereby incorporated into the IFB by reference.

3. Acknowledgment

Bidders must acknowledge receipt of this Addendum No. 6 on the Bid Cover Page / Signature Affidavit. Failure to acknowledge this Addendum may result in rejection of the bid as non-responsive.

Issued By: _____

Benjamin Leischner, A.A.E
Executive Director
Metropolitan Airport Authority of Rock Island County, Illinois

Dated: March 10, 2026

SCHEDULE E

DESIGN LOAD CRITERIA:

1. AS REQUIRED BY ROCK ISLAND COUNTY, IL, ENGINEERING DESIGN IS BASED ON AND ACCORDANCE WITH THE FOLLOWING CODE(S):
   - A. INTERNATIONAL BUILDING CODE - 2012
2. DESIGN LOADS:
   - A. SUPERIMPOSED ROOF DEAD LOAD ..... BY METAL BUILDING MNFR.
     FLOOR LIVE LOAD ..... 250 PSF
   - B. SNOW LOAD:
     1) GROUND SNOW LOAD (Pg) ..... 20 PSF
     2) SNOW EXPOSURE FACTOR (Ce) 1.0
     3) SNOW IMPORTANCE FACTOR 1.00
     4) THERMAL FACTOR (Ct) 1.0
   - C. WIND LOAD:
     1) BASIC WIND SPEED (3 SEC GUST) ..... 115 MPH
     2) WIND IMPORTANCE FACTOR 1.0
     3) BUILDING CATEGORY II
     4) WIND EXPOSURE C
     5) INTERNAL PRESSURE COEFFICIENT ..... ±0.18
   - D. SEISMIC:
     1) SEISMIC DESIGN CATEGORY ..... B
     2) SEISMIC IMPORTANCE FACTOR 1.00
     3) SPECTRAL RESPONSE COEFFICIENTS
        Ss 0.19
        S1 ..... 0.085
     4) SITE CLASS ..... D
     5) BASIC SEISMIC FORCE RESISTING SYSTEMS DESCRIPTION:
        STEEL SYSTEMS NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE
        RESPONSE MODIFICATION FACTOR (R) USED 3.0

GENERAL REQUIREMENTS:

1. DEFINITIONS:
   - A. ENGINEER: REFERENCES OF THE STRUCTURAL DRAWINGS TO 'ENGINEER' MEAN THE STRUCTURAL ENGINEER OF RECORD. OTHER ENTITIES ARE SPECIFICALLY NOTED AS "CONTRACTORS ENGINEER", "MECHANICAL ENGINEER", ETC.
2. UNDERGROUND UTILITIES: LOCATE EXISTING UTILITIES, AND NOTIFY ENGINEER OF EXISTING UTILITIES OR SUBGRADE CONDITIONS WHICH INTERFERE WITH WORK.
3. EXISTING STRUCTURES:
   - A. CONTRACT DOCUMENTS HAVE BEEN PREPARED USING AVAILABLE DRAWINGS AND SITE OBSERVATION AS PERMITTED BY ACCESS RESTRICTIONS DURING DESIGN.
   - B. DURING CONSTRUCTION, THE CONTRACTOR MAY ENCOUNTER EXISTING CONDITIONS WHICH ARE NOT NOW KNOWN OR ARE AT VARIANCE WITH PROJECT DOCUMENTATION (DISCOVERY). CONTRACTOR SHALL NOTIFY THE ENGINEER OF ALL CONDITIONS NOT PER THE CONTRACT DOCUMENTS. EXAMPLES INCLUDE:
     - SIZES OR DIMENSIONS OTHER THAN THOSE SHOWN.
     - DAMAGE OR DETERIORATION TO MATERIALS OR COMPONENTS.
     - ITEMS NOTED AS EXISTING ON THE DRAWINGS BUT NOT FOUND IN THE FIELD.
   - C. PREPARE DIMENSIONAL DRAWINGS OF ALL DISCOVERED ITEMS.
   - D. CONTRACTOR SHALL FIELD VERIFY ALL EXISTING STRUCTURAL CONDITIONS PRIOR TO SUBMITTING SHOP DRAWINGS.
   - E. CONTRACTOR SHALL MAKE ALLOWANCE FOR THE RESOLUTION OF SUCH DISCOVERIES IN THE CONSTRUCTION SCHEDULE.
4. USE OF DRAWINGS:
   - A. DO NOT SCALE DRAWINGS.
   - B. WHERE DISCREPANCIES OCCUR BETWEEN PLANS, DETAILS AND GENERAL NOTES, THE MORE STRINGENT REQUIREMENTS SHALL GOVERN. DETAILS ON DRAWINGS TAKE PRECEDENCE OVER GENERAL NOTES AND TYPICAL DETAILS. DETAILS NOTED TYPICAL APPLY TO ALL SIMILAR CONDITIONS. WHERE NO SPECIFIC DETAILS ARE SHOWN, CONSTRUCTION SHALL CONFORM TO SIMILAR WORK ELSEWHERE ON THE PROJECT.
5. TEMPORARY CONDITIONS:
   - A. THE STRUCTURE IS DESIGNED TO FUNCTION AS A UNIT UPON COMPLETION. THE CONTRACTOR IS RESPONSIBLE FOR FURNISHING ALL TEMPORARY BRACING AND/OR SUPPORT THAT MAY BE REQUIRED AS THE RESULT OF THE CONTRACTOR'S CONSTRUCTION METHODS AND/OR SEQUENCES.
   - B. CONTRACTOR'S CONSTRUCTION AND/OR ERECTION SEQUENCES SHALL RECOGNIZE AND CONSIDER THE EFFECTS OF THERMAL MOVEMENTS OF STRUCTURAL ELEMENTS DURING THE CONSTRUCTION PERIOD.
6. SUBMITTALS AND SUBSTITUTIONS:
   - A. SUBMITTALS:
     1) IF THE CONTRACTOR REQUESTS A CHANGE FROM THE STRUCTURAL DRAWINGS, IT SHALL BE APPROVED AND DESIGNED BY THE ENGINEER OF RECORD PRIOR TO SUBMITTING SHOP DRAWINGS. VARIATION SHALL BE INDICATED ON THE SHOP DRAWINGS. THE CONTRACTOR SHALL COMPENSATE CARR ENGINEERING, LLC FOR CHANGES.
     2) CONSTRUCTION DOCUMENTS SHALL NOT BE REPRODUCED FOR USE IN SUBMITTALS.
   - B. SUBSTITUTIONS: ENGINEER'S APPROVAL SHALL BE SECURED FOR ALL SUBSTITUTIONS.
   - C. NON-CONFORMANCE: NOTIFY ENGINEER OF CONDITIONS NOT CONSTRUCTED PER CONTRACT DOCUMENTS PRIOR TO PROCEEDING WITH CORRECTIVE WORK. SUBMIT PROPOSED REPAIR TO THE ENGINEER FOR ACCEPTANCE. CONTRACTOR SHALL COMPENSATE CARR ENGINEERING, LLC FOR DESIGN OF THE REPAIR.
7. OSHA STANDARDS:
   - A. THE STRUCTURE IS DESIGNED TO FUNCTION AS A UNIT UPON COMPLETION. NOTHING SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE CONSTRUED AS ELIMINATING THE NEED FOR THE CONTRACTOR TO COMPLY WITH ALL OSHA REQUIREMENTS. THE CONTRACTOR SHALL ADD ALL NECESSARY BOLTS, ANCHOR BOLTS, STIFFENER PLATES, STABILIZER PLATES, BRIDGING, BRACING, BEARING SEATS, COLUMN SPLICES ETC., AS WELL AS CLOSURES FOR OPENINGS.
   - B. WASHERS OR RINGS SHALL BE WELDED TO STEEL COLUMNS TO PROVIDE FOR SAFETY CABLES, DO NOT PLACE HOLES IN COLUMNS WITHOUT APPROVAL OF THE STRUCTURAL ENGINEER.
   - C. WHERE THE STRUCTURAL DRAWINGS APPEAR TO CONFLICT WITH OSHA REQUIREMENTS, THE STRUCTURAL DRAWINGS REPRESENT FINAL CONDITIONS ONLY; THE CONTRACTOR SHALL ADD ALL ERECTION FRAMING THAT MAY BE NECESSARY TO COMPLY WITH OSHA.
8. CONSTRUCTION ENGINEERING:

   THE STRUCTURE DEFINED IN THE CONTRACT DOCUMENTS HAS BEEN DESIGNED ONLY FOR LOADS ANTICIPATED ON THE STRUCTURE DURING ITS SERVICE LIFE. PROVIDE ALL REQUIRED ENGINEERING AND OTHER MEASURES TO ACHIEVE THE MEANS, METHODS AND SEQUENCES OF WORK. SUCH ENGINEERING MAY INCLUDE, BUT IS NOT LIMITED TO:
   - LAYOUT.
   - DESIGN OF FORMWORK, SHORING AND RESHORING.
   - DESIGN OF CONCRETE MIXES.
   - ERECTION PROCEDURES WHICH ADDRESS STABILITY OF THE FRAME DURING CONSTRUCTION OF STEEL.
   - WELD PROCEDURES.
   - DESIGN OF TEMPORARY BRACING OF WALLS FOR WIND, SEISMIC, OR SOIL LOADS.
   - SURVEYING TO VERIFY CONSTRUCTION TOLERANCES.
   - EVALUATION OF TEMPORARY LOADS ON STRUCTURE DUE TO EQUIPMENT AND MATERIALS DURING CONSTRUCTION.
   - STRUCTURAL ENGINEERING TO RESIST ANY OTHER LOADS NOT IDENTIFIED ON DESIGN DRAWINGS.
9. COORDINATION:
   - A. STRUCTURAL DRAWINGS RE NOT STAND-ALONE DOCUMENTS AND ARE INTENDED TO BE USED IN CONJUNCTION WITH CIVIL, ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND DRAWINGS FROM OTHER DISCIPLINES. THE CONTRACTOR SHALL COORDINATE ALL REQUIREMENTS OF THE CONTRACT DOCUMENTS INTO SHOP DRAWINGS AND WORK.
   - B. COORDINATE DIMENSIONS OF ALL OPENINGS, BLOCKOUTS, DEPRESSIONS, ETC. WITH DRAWINGS FROM OTHER DISCIPLINES, PROJECT SHOP DRAWINGS, AND FIELD CONDITIONS PRIOR TO SHOP DRAWINGS SUBMITTAL.
10. SPECIAL INSPECTION:
    - A. SPECIAL INSPECTION SHALL BE PROVIDED PER IBC 2012 CHAPTER 17. THE LIST BELOW IS A SUMMARY OF ITEMS REQUIRING INSPECTION BY THE IBC:

      SEE TABLE ON SHEET S101

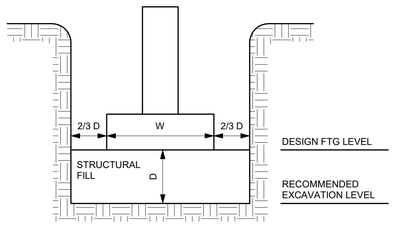
EARTHWORK NOTES:

1. REFER TO ARCHITECTURAL DRAWINGS FOR BUILDING LOCATION ON THE SITE, FLOOR ELEVATION AND OTHER PERTINENT SITE AND BUILDING INFORMATION.
2. ELEVATIONS GIVEN THUS ARE TO THE TOP OF FOOTINGS, SLABS, BEAMS, JOISTS, ETC. WITH REFERENCE TO THE FINISHED FLOOR ELEVATION = (0'-0").
3. A GEOTECHNICAL REPORT IS NOT AVAILABLE FOR THIS SITE. ADDITIONAL TEST BORINGS AND OTHER EXPLORATORY OPERATIONS MAY BE MADE BY THE CONTRACTOR WITH APPROVAL FROM THE OWNER. DESIGN VALUES FROM THE 2012 IBC ARE AS FOLLOWS:
   - A. ALLOWABLE SOIL BEARING FOR FOOTING ..... 2500 PSF
   - B. MINIMUM SOIL SUBGRADE MODULUS REQUIRED ..... 125 PCI
4. THE TOP 9" (MIN.) OF THE EXISTING GRADE IS TO BE SCARIFIED AND RECOMPACTED TO MEET OR EXCEED THE REQUIREMENT GIVEN IN THE GEOTECHNICAL REPORT.
5. UNLESS NOTED OTHERWISE, NEW FOOTINGS SHALL CENTER UNDER THE WALLS, PIERS OR COLUMNS.
6. WATER SHALL NOT BE ALLOWED TO ACCUMULATE IN THE EXCAVATION. PROVIDE PUMPS, IF NECESSARY, AND OPERATE NIGHT AND DAY, WHENEVER REQUIRED.
7. THE BASE OF ALL FOOTING EXCAVATIONS SHALL BE OBSERVED BY THE GEOTECHNICAL ENGINEER PRIOR TO PLACEMENT OF CONCRETE.
8. BACKFILL PLACED WITHIN THE BUILDING AREA SHALL BE WITH SOIL THAT IS APPROVED BY THE GEOTECHNICAL ENGINEER. PLACE IN LIFTS AND COMPACT IN STRICT COMPLIANCE WITH RECOMMENDATIONS OF THE GEOTECHNICAL ENGINEER. PLACE IN LOOSE LIFTS OF 8 INCHES OR LESS AND COMPACT. FILL SHOULD BE FREE OF FAT CLAYS AND ORGANIC SOIL (UNIFIED SOIL CLASSIFICATIONS CH, OL, OH, MH OR PT), FROZEN MATERIAL, RUBBLE, DEGRADABLE MATERIAL, CHEMICAL CONTAMINANTS AND STONES THAT INTERFERE WITH COMPACTION OR COMPACTION TESTING. IN GENERAL, AT LEAST 70% OF THE FILL SHOULD PASS A 3/4-INCH SIEVE, THE OVER-SIZE LIMIT FOR THE ASTM D698 MAXIMUM DENSITY TEST.
9. FILL SHOULD BE PLACES ON STABLE, UNFROZEN SURFACES AND IN NEARLY HORIZONTAL LIFTS THAT HAVE THICKNESSES THAT ARE COMPATIBLE WITH THE COMPACTION EQUIPMENT USED, BUT GENERALLY NOT OVER EIGHT INCHES. ALL FILL OTHER THAN COARSE AGGREGATES SHOULD BE COMPACTED TO AT LEAST 95% OF ITS MAXIMUM DRY DENSITY DETERMINED BY ASTM D698, TEST METHOD FOR LABORATORY COMPACTION CHARACTERISTICS OF SOIL USING STANDARD EFFORT (12,400 FT-LB/CUBIC FOOT). COMPACTION USUALLY REQUIRES ADJUSTMENTS OF FILL MOISTURE CONTENTS TO WITHIN THREE PERCENTAGE POINTS OF THE ASTM D698 OPTIMUM MOISTURE. COARSE AGGREGATES SHOULD BE VIBRATORY-COMPACTED TO INTERLOCK THE PARTICLES TO 98% STANDARD PROCTOR PER ASTM D698.

CONCRETE NOTES:

1. CONCRETE WORK SHALL CONFORM TO ALL REQUIREMENTS OF ACI 301-99, SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS, EXCEPT AS MODIFIED BY SUPPLEMENTAL REQUIREMENTS CONTAINED IN THE FOLLOWING NOTES.
2. UNLESS OTHERWISE DETAILED OR NOTED, REINFORCING SHALL BE IN ACCORDANCE WITH "THE ACI DETAILING MANUAL, SP-66."
3. SUBMIT SHOP DRAWINGS FOR FABRICATION AND PLACEMENT OF CONCRETE REINFORCING. SHOW PLAN VIEW OF SLAB, INCLUDE SCHEDULES AND DIAGRAMS OF BENT BARS AND SHOW ARRANGEMENT OF CONCRETE REINFORCING. ENGINEER'S REVIEW OF SHOP DRAWINGS WILL BE FOR COMPLIANCE WITH DESIGN REQUIREMENTS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR VERIFYING DIMENSIONS AND QUANTITIES RELATIVE TO REINFORCING STEEL.
4. ALL CONCRETE TESTING SHALL BE PERFORMED UNDER THE DIRECT SUPERVISION OF A QUALIFIED ENGINEER WHO IS LICENSED TO PRACTICE IN THE STATE OF ILLINOIS. THE TESTING AGENCY SHALL MEET REQUIREMENTS CONTAINED IN ASTM E329. SEE SPECIFICATIONS FOR TESTING REQUIREMENTS.
5. PREPARE DESIGN MIXES FOR EACH TYPE AND STRENGTH OF CONCRETE BY EITHER LABORATORY TRIAL BATCH OR FIELD EXPERIENCE METHODS AS SPECIFIED IN ACI 301.
   - A. TRIAL BATCH METHOD: IF TRIAL BATCHES ARE USED, AND INDEPENDENT TESTING LABORATORY, APPROVED BY THE ARCHITECT, SHALL PREPARE MIX DESIGNS. THE MIX DESIGN SHALL BE PROPORTIONED TO ACHIEVE AN AVERAGE STRENGTH OF 1200 PSI HIGHER THAN THE SPECIFIED STRENGTH (F'C').
   - B. FIELD EXPERIENCE METHOD: IF THE FIELD EXPERIENCE METHOD IS SELECTED, THE PROPOSED MIX DESIGN MUST BE ACCOMPANIED BY THE COMPLETE STANDARD DEVIATION ANALYSIS.
   - C. SLUMP SHALL BE 4" MAXIMUM, 2" MINIMUM FOR STRUCTURAL SLABS.
6. READY-MIX CONCRETE SHALL BE IN COMPLIANCE WITH REQUIREMENTS OF ASTM C94, AND AS HEREIN SPECIFIED.
7. COMPRESSIVE STRENGTH: STRENGTH REQUIREMENTS AT 28 DAYS AND THE LOCATION OF EACH TYPE OF CONCRETE REQUIRED FOR THIS PROJECT SHALL BE AS FALLOWS:

TYPE	INTENDED USE	28 DAY MAX STRENGTH	MAX WATER/CEMENT RATIO INCLUDING FLY ASH	MIN CEMENT MATERIAL INCLUDING FLY ASH	MAX AGGREGATE SIZE	CEMENT TYPE	SLUMP LIMITS (+1", -2")
1	SLAB-ON -GRADE	4 KSI	0.50	520 LB/CU. YD	3/4 IN	I OR II	4
2	FOOTINGS GRADE BEAMS AND WALLS	4 KSI	0.50	520 LB/CU. YD	1 IN	I OR II	4



1 **TYP FOOTING OVER EXCAVATION**
3/4" = 1'-0"

METAL BUILDING CRITERIA:

1. FOUNDATION DESIGN IS BASED ON PRELIMINARY DESIGN DRAWINGS RECEIVED FROM BUTLER MANUFACTURING. FINAL DESIGN REACTIONS SUPPLIED BY THE SELECTED METAL BUILDING MANUFACTURER MAY DIFFER FROM PRELIMINARY ESTIMATED REACTIONS. BASED ON FINAL REACTIONS SUPPLIED, MODIFICATIONS TO THE FOUNDATION PLAN MAY BE REQUIRED. GENERAL CONTRACTOR SHALL INCLUDE IN THEIR BID AN ADD AND DEDUCT PRICE PER CUBIC YARD OF CONCRETE WORK INSTALLED IN CASE SUCH CHANGES ARE REQUIRED.
2. METAL BUILDING MANUFACTURER SHALL ENGAGE A PROFESSIONAL ENGINEER LICENSED TO PRACTICE STRUCTURAL ENGINEERING IN THE STATE OF ILLINOIS TO DESIGN THE METAL BUILDING AND TO SUBMIT STAMPED AND SEALED DRAWINGS AND STRUCTURAL CALCULATIONS FOR THE METAL BUILDING.
3. FOUNDATION DESIGN IS BASED ON THE METAL BUILDING COLUMNS HAVING PINNED BASES AND TRANSFERRING NO MOMENTS TO THE FOUNDATIONS.
4. ALL EXTERNAL LATERAL LOAD RESISTANCE AND STABILITY OF THE METAL BUILDING IN THE COMPLETED STRUCTURE IS PROVIDED BY MOMENT FRAMES AND TENSION ONLY ROD BRACING. THE VERTICAL LATERAL MEMBERS CARRY THE APPLIED LATERAL LOADS TO THE BUILDING FOUNDATION. THE TENSION ONLY ROD BRACING AT THE ROOF SERVES AS THE HORIZONTAL DIAPHRAGM THAT DISTRIBUTES THE LATERAL WIND AND SEISMIC FORCES HORIZONTALLY TO THE VERTICAL LATERAL MEMBERS. ALL LATERAL RESISTING MEMBERS ARE BY THE METAL BUILDING MANUFACTURER.
5. METAL BUILDING MANUFACTURER IS SOLELY RESPONSIBLE FOR THE DESIGN AND MANUFACTURER OF THE METAL BUILDING. THE BUILDING MUST BE DESIGNED TO MEET THE LOADING INDICATED IN THE DRAWINGS AND THE DESIGN CRITERIA INDICATED ABOVE.
6. METAL BUILDING MANUFACTURER IS TO COORDINATE WITH ARCHITECTURAL DRAWINGS FOR WINDOW AND DOOR OPENINGS WHEN DESIGNING THE LATERAL SYSTEM.
7. SEE METAL BUILDING DRAWINGS FOR ANCHOR ROD MATERIAL GRADE, ANCHOR ROD DIAMETERS, QUANTITIES AND LAYOUT.

REINFORCEMENT STEEL:

1. FOR CAST-IN-PLACE CONCRETE THE FOLLOWING MINIMUM CONCRETE COVER SHALL BE PROVIDED FOR REINFORCEMENT UNLESS NOTED OTHERWISE:

CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH	3 INCHES
CONCRETE EXPOSED TO EARTH OR WEATHER	
NO. 6 BARS OR LARGER	2 INCHES
NO. 5 BARS OR SMALLER	1 1/2 INCHES
SLABS, WALLS, JOISTS NOT EXPOSED TO WEATHER OR IN CONTACT WITH EARTH	
NO. 14 AND NO. 18 BARS	1 1/2 INCHES
NO. 11 BARS OR SMALLER	3/4 INCHES
BEAMS AND COLUMNS NOT EXPOSED TO WEATHER OR IN CONTACT WITH EARTH	1 1/2 INCHES

2. DIMENSIONS OF CONCRETE COVER FOR REINFORCEMENT INDICATED ON DRAWINGS ARE TO OUTERMOST REINFORCING BARS. FOR BEAMS OR COLUMNS WITH STIRRUPS OR TIES, CLEAR COVER INDICATED IS TO STIRRUPS OR TIES.
3. BAR SPLICES: SPLICE REINFORCING WHERE INDICATED ON THE DRAWINGS. ALL SPLICES SHALL BE CLASS 'B' AS DEFINED IN ACI 318. IF SPLICE LENGTH IS NOT GIVEN ON THE DRAWINGS, PROVIDE LAP LENGTHS (IN INCHES) AS FOLLOWS:

	3000 PSI CONC		4000 PSI CONC	
BAR SIZE	OTHER	TOP	OTHER	TOP
#3	22	28	19	25
#4	29	38	25	33
#5	36	47	31	41
#6	43	56	37	49
#7	63	81	54	71
#8	72	93	62	81
#9	81	105	70	91
#10	91	118	79	102
#11	101	131	87	114

   LAP LENGTHS ASSUME CLEAR SPACING BETWEEN BARS OF 2 BAR DIAMETERS, AND A MINIMUM COVER OF 1 BAR DIAMETER. FOR DEVELOPMENT LENGTHS, DIVIDE BY 1.3. TOP BARS ARE DEFINED AS HORIZONTAL BARS WITH MORE THAN 1'-0" OF FRESH CONCRETE BELOW.
4. SUBMIT SHOP DRAWINGS SHOWING REINFORCING STEEL QUANTITIES AND PLACEMENT. REINFORCING STEEL DESIGNATIONS ON SHOP DRAWINGS SHALL BE INCH-POUND SIZES.
5. ALL REINFORCING STEEL SHALL BE DETAILED AND PLACED IN CONFORMANCE WITH THE AMERICAN CONCRETE INSTITUTE "DETAILS AND DETAILING OF REINFORCED CONCRETE" (ACI 315) EXCEPT AS OTHERWISE SHOWN, NOTED OR SPECIFIED.
6. PROVIDE ADEQUATE TIES FOR ALL REINFORCING BARS AND STIRRUPS IN CONCRETE SLABS AND BEAMS. REINFORCING BARS TO BE HELD AT CORRECT DISTANCE FROM FORMS BY ADEQUATE CONCRETE BLOCKS, STEEL CHAIRS OR TIES. ALL REINFORCING BARS, ANCHOR BOLTS AND OTHER CONCRETE INSERTS SHALL BE SECURED IN POSITION WITH TIES OR WELDS PRIOR TO PLACING CONCRETE.
7. UNLESS NOTED OTHERWISE, SUPPORTS FOR REINFORCEMENT SHALL HAVE CLASS 2 PROTECTION AS DEFINED IN THE CRSI MANUAL OF STANDARD PRACTICE.
8. SUPPORTS FOR COATED REINFORCEMENT SHALL HAVE CLASS 1 PROTECTION AS DEFINED IN THE CRSI MANUAL OF STANDARD PRACTICE.
9. CONTINUOUS REINFORCING SHALL BE LAPPED AS FOLLOWS UNLESS NOTED OTHERWISE:
   TOP BARS MIDSPAN
   BOTTOM BARS DIRECTLY OVER SUPPORT
10. DOWELS BETWEEN FOOTINGS AND WALLS OR COLUMNS SHALL BE THE SAME GRADE, SIZE AND SPACING OR NUMBER AS THE VERTICAL REINFORCING, RESPECTIVELY, UNLESS NOTED OTHERWISE.
11. REINFORCING BARS SHALL BE BENT COLD, AND NO METHOD OF FABRICATION SHALL BE USED WHICH WOULD BE INJURIOUS TO THE MATERIAL. HEATING OF BARS FOR BENDING IS NOT PERMITTED.

CARR ENGINEERING
STRUCTURAL SERVICES

CARR ENGINEERING, LLC
ILLINOIS DESIGN FIRM LIC No.
184.007877-0003

3294 Fields Dr.
Bettendorf, IA 52722
(563) 503-6100

NO.	DESCRIPTION	DATE
1	PERMIT SET	11.04.25

QUAD CITY INTERNATIONAL AIRPORT
METROPOLITAN AIRPORT AUTHORITY
ROCK ISLAND CO.

KIRK HANGAR BUILDING

MOLINE, ILLINOIS

Project number	25-134
Date	11/04/2025
Drawn by	WDH
Checked by	CMC

GENERAL NOTES

STATE OF ILLINOIS
CHAD M. CARR
081-006490
LICENSED STRUCTURAL ENGINEER

I hereby certify that this engineering document was prepared by me or under my direct personal supervision and that I am a duly licensed Structural Engineer under the laws of the State of Illinois.

Signature 11-04-2025 Date

Chad M. Carr
License No. 081-006490
My license renewal date is November 30, 2026
Pages or sheets covered by this seal: S100 - S301

Carr Engineering LLC
Illinois Design Firm License: #184.007877-0003

S100

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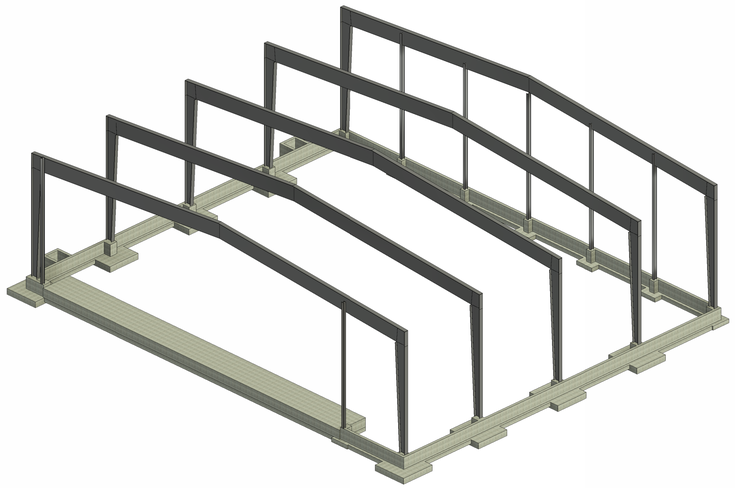
STRUCTURAL COMPONENT TESTING AND INSPECTION

1. THE FOLLOWING TESTING AND INSPECTION OF STRUCTURAL COMPONENTS IS REQUIRED AS DETAILED IN CHAPTER 17 OF THE 2012 INTERNATIONAL BUILDING CODE (IBC).
2. SEE MECHANICAL, PLUMBING AND ELECTRICAL SPECIFICATIONS FOR TESTING AND INSPECTION REQUIREMENTS OF NON-STRUCTURAL COMPONENTS.
3. WORK PERFORMED ON THE PREMISES OF A FABRICATOR APPROVED BY THE BUILDING OFFICIAL PER SECTION 1704.2.5.1 OF CHAPTER 17 OF THE 2012 INTERNATIONAL BUILDING CODE NEED NOT BE TESTED AND INSPECTED PER THE TABLE BELOW. THE FABRICATOR SHALL SUBMIT A CERTIFICATE OF COMPLIANCE THAT THE WORK HAS BEEN PERFORMED IN ACCORDANCE WITH THE APPROVED PLANS AND SPECIFICATIONS TO THE BUILDING OFFICIAL AND THE ARCHITECT AND ENGINEER OF RECORD.
4. DUTIES OF THE SPECIAL INSPECTION AGENCY (IBC CHAPTER 17):
   - A. PERFORM ALL TESTING AND INSPECTION REQUIRED PER APPROVED TESTING AND INSPECTION PROGRAM.
   - B. FURNISH INSPECTION REPORTS TO THE BUILDING OFFICIAL AND THE OWNER.
   - C. SUBMIT A FINAL SIGNED REPORT STATING WHETHER THE WORK REQUIRING SPECIAL INSPECTION WAS, TO THE BEST OF THE SPECIAL INSPECTION AGENCY'S KNOWLEDGE, IN CONFORMANCE WITH THE APPROVED PLANS AND SPECIFICATIONS.

	CONTINUOUS	PERIODIC	REFERENCE D STANDARD	IBC REFERENCE
FOUNDATION PREPARATION				
VERIFY MATERIALS BELOW SHALLOW FOOTINGS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY.		X		1705.6
VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL.		X		1705.6
PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS.		X		1705.6
VERIFY USE OF PROPER MATERIALS, DENSITIES, AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF COMPACTED FILL.	X			1705.6
PRIOR TO PLACEMENT OF COMPACTED FILL, OBSERVE SUBGRADE AND VERIFY THAT THE SITE HAVE BEEN PROPERLY PREPARED.		X		1705.6
CONCRETE AND CONCRETE PLACEMENT				
INSPECT FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED.		X	ACI 318: 26.11.1.2(b)	
INSPECTION OF REINFORCING STEEL, INCLUDING PRESTRESSING TENDONS, AND PLACEMENT.		X	ACI 318: CHG. 20, 25.2, 25.3, 26.6.1-26.6.3	1908.4
INSPECTION OF ANCHORS POST-INSTALLED INSTALLED IN HARDENED CONCRETE MEMBERS.				
A. ADHESIVE ANCHORS INSTALLED IN HORIZONTALLY OR UPWARD INCLINED ORIENTATIONS TO RESIST SUSTAINED LOADS.	X		ACI 318: 17.8.2.4(b)	
VERIFYING USE OF REQUIRED DESIGN MIX.		X	ACI 318: CH. 19, 26.4.3, 26.4.4	1904.1, 1904.2 1908.2, 1908.3
PRIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE.	X		ACI 318: 26.4, 26.12	1908.10
INSPECTION OF CONCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES.	X		ACI 318: 26.5	1908.6, 1908.7, 1908.8
F_F AND F_L SLAB ON GRADE FLATNESS TESTING			ASTM E1155	
STRUCTURAL STEEL			REFERENCE D STANDARD	IBC REFERENCE
1. MATERIAL VERIFICATION OF HIGH-STRENGTH BOLTS, NUTS, AND WASHERS:				
A. IDENTIFICATION MARKINGS TO CONFORM TO ASTM STANDARDS SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS.		X	APPLICABLE ASTM MATERIAL STANDARDS: AISC 360, SECTION A3.3	
B. MANUFACTURER'S CERTIFICATE OF COMPLIANCE REQUIRED.		X		
2. INSPECTION OF HIGH-STRENGTH BOLTING:				
A. SNUG TIGHT JOINTS.		X	AISC 360, SECTION M2.5	
STEEL DECK			REFERENCE D STANDARD	IBC REFERENCE
1. MATERIAL VERIFICATION OF STRUCTURAL STEEL				
A. IDENTIFICATION MARKINGS TO CONFORM TO ASTM STANDARDS SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS.		X	APPLICABLE ASTM MATERIAL STANDARDS	
B. MANUFACTURER'S CERTIFIED TEST REPORTS.		X		
2. INSPECTION OF WELDING:				
A. FLOOR AND ROOF DECK WELDS.		X	AW D1.3	



1 **ISOMETRIC VIEW**

ALL STEEL FRAMING IS TO BE DESIGNED BY THE METAL BUILDING MANUFACTURER

ABBREVIATIONS

APPROX	APPROXIMATE, APPROXIMATLY
ARCH	ARCHITECT, -URAL, -URE
BP	BASE PLATE
BLDG	BUILDING
B.D.	BOTTOM OF
BRG	BEARING
C/C	CENTER TO CENTER
CFMS	COLD FORMED METAL STEEL
CLR	CLEAR, -(ANCE)
CMU	CONCRETE MASONRY UNIT
CONC	CONCRETE
CONT	CONTINUOUS
D	DEPTH
DAS	DEFORMED ANCHOR STUD
DEG	DEGREE
DEMO	DEMOLITION
DIM	DIMENSION
DL	DEAD LOAD
DWG	DRAWING
EA	EACH
EF	EACH FACE
EMBED	EMBEDDED
EOD	EDGE OF DECK
EOS	EDGE OF SLAB
EQ	EQUAL
EXIST	EXISTING
(E)	EXISTING STRUCTURAL MEMBER
EXP	EXPANSION
EXT	EXTERIOR
f'c	CONCRETE COMPRESSIVE MEMBER
FND	FOUNDATION
FIN	FINISH
FLR	FLOOR
FR	FRAMING
FT	FOOT
FTG	FOOTING
GA	GAUGE
GALV	GALVANIZED
GC	GENERAL CONTRACTOR
GYP	GYPSUM BOARD
HORIZ	HORIZONTAL
HDAB	HEAD ANCHOR BOLT
HDAS	HEAD ANCHOR STUD
I.F.	INSIDE FACE
IN.	INCH
JST	JOIST
LL	LIVE LOAD
LLH	LONG LEG HORIZONTAL
LLV	LONG LEG VERTICAL
MAX	MAXIMUM
MIN	MINIMUM
MISC	MISCELLANEOUS
(N)	NEW STRUCTURAL MEMBER
NIC	NOT IN CONTRACT
NTS	NOT TO SCALE
OC	ON CENTER
O.F.	OUTSIDE FACE
OPNG	OPENING
OPP	OPPOSITE
PC	PRECAST
PL	PLATE
PSF	POUNDS PER SQUARE FOOT
PSI	POUNDS PER SQUARE INCH
R	RADIUS
REINF	REINFORCING, -ED, -MENT
REQ'D	REQUIRED
SIM	SIMILAR
SP	SPACE(S)
SPEC	SPECIFICATION(S)
SPEC'D	SPECIFIED
STD	STANDARD
STIFF	STIFFENER
T.O.	TOP OF
TYP	TYPICAL
U.N.O.	UNLESS NOTED OTHERWISE
VERT	VERTICAL
WWF	WELDED WIRE FABRIC
@	AT
Ø OR DIA	DIAMETER
# OR NO.	NUMBER

SYMBOLS LEGEND

DESCRIPTION	SYMBOL	DESCRIPTION	SYMBOL
STEP UP =		ELEVATION =	T/X EL (XXX'-X")
SLOPE UP =		REVISION CLOUD =	
MOMENT CONNECTION =		NORTH ARROW =	NORTH
DETAIL SECTION CUT =	DTL# SHT#	GRADE =	
BUILDING SECTION CUT =	DTL# SHT#	STRUCTURAL FILL =	
CONCRETE IN SECTION =		SUBGRADE =	
MASONRY =		BRACE =	
FOOTING SIZE =	F-X	WELDED WIRE FABRIC =	
COLUMN =	C-X	FOOTING STEP =	(-X'-X") (-X'-X")

CARR ENGINEERING
STRUCTURAL SERVICES

CARR ENGINEERING, LLC
ILLINOIS DESIGN FIRM LIC No.
184.007877-0003

3294 Fields Dr.
Bettendorf, IA 52722
(563) 503-6100

NO.	DESCRIPTION	DATE
1	PERMIT SET	11.04.25

QUAD CITY INTERNATIONAL AIRPORT
METROPOLITAN AIRPORT AUTHORITY
ROCK ISLAND CO.

KIRK HANGAR BUILDING

MOLINE, ILLINOIS

Project number	25-134
Date	11/04/2025
Drawn by	WDH
Checked by	CMC

NOTES & SYMBOLS

S101

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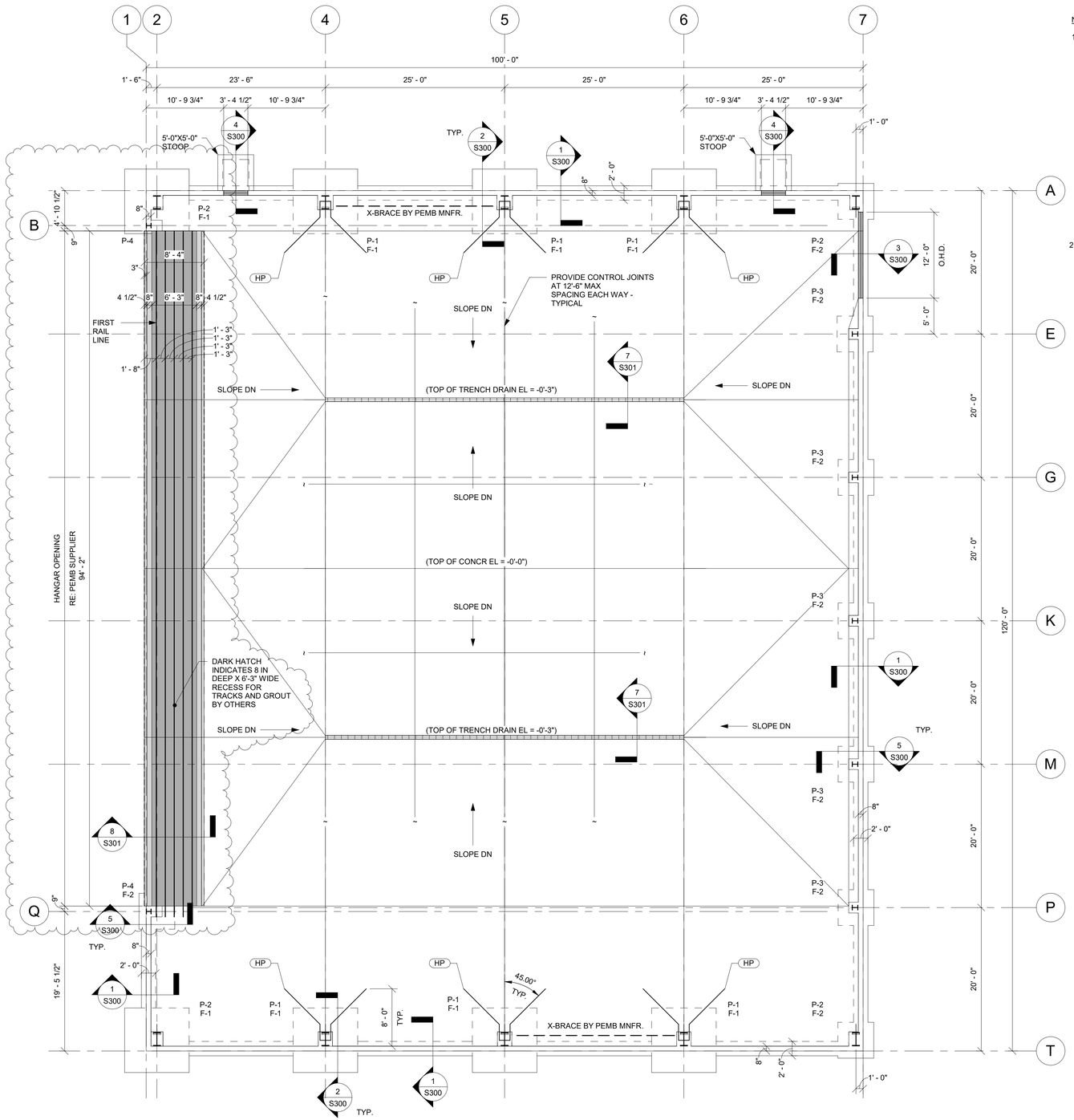


MAIN FLOOR FOUNDATION PLAN

1/8" = 1'-0"

NOTES:

1. SLAB-ON-GRADE:
   - A. GRIDLINES ARE LOCATED AT OUTSIDE FACE OF FOUNDATION WALL OR COLUMN CENTERS UNLESS NOTED OTHERWISE.
   - B. SLAB-ON-GRADE TO BE 8 INCH THICK NORMAL WEIGHT CONCRETE ON 14 INCH (MIN.) OF **EXISTING ENGINEERED GRANULAR BASE**. **(CRUSHED AGGREGATE)** RE: *5/S301*.
   - C. REINFORCE 8 INCH SLAB-ON-GRADE WITH FORTA FERRO FIBER ADMIXTURE (DOSAGE RATE 5.0 LB/CY), (ALTERNATE SLAB REINFORCING #4 @ 18" O.C. CENTERED.
   - D. TOP OF SLAB-ON-GRADE IS AT ELEVATION = (0'-0") UNLESS NOTED OTHERWISE.
   - E. PLACE CONTROL OR CONSTRUCTION JOINTS AT 12'-6" ON CENTER MAXIMUM. USE DOWEL BASKETS AT ALL CONTROL JOINTS. SEE *6/S301*.
   - F. REFERENCE ARCHITECTURAL DRAWINGS FOR SLAB SLOPES AND DEPRESSIONS NOT SHOWN ON STRUCTURAL DRAWINGS.
2. FOOTINGS:
   - A. FOOTINGS ARE SHOWN ON PLAN THUS: F-X. REFERENCE DETAILS AND FOOTING SCHEDULE FOR SIZES AND REINFORCING.
   - B. ALL COLUMN FOOTINGS SHALL BE CENTERED ON PIERS UNLESS OTHERWISE DIMENSIONED.
   - C. TOP OF EXTERIOR FOOTING IS AT ELEVATION (-3'-0") UNLESS NOTED OTHERWISE.
   - D. SEE FOOTING OVER-EXCAVATION DETAIL THIS SHEET WHERE REQUIRED.
3. PIERS:
   - A. PIER TYPE NOTED ON PLAN THUS: P-X. SEE SHEET S300 FOR DETAILS.
   - B. TOP OF PIER ELEVATION TO BE (0'-0") UNLESS NOTED OTHERWISE.
4. FOUNDATIONS:
   - A. CONSULT WITH OWNER'S GEOTECHNICAL ENGINEER AND SEE PROJECT'S REPORT OF SUBSURFACE EXPLORATION AND GEOTECHNICAL ENGINEERING REQUIREMENTS, SPECIFIC FOUNDATION SUBGRADE PREPARTION REQUIREMENTS, UNDER-SLAB DRAINAGE, SITE DEWATERING AND OTHER EARTH WORK REQUIREMENTS.
5. COLUMNS:
   - A. ALL COLUMNS, BASEPLATES AND ANCHOR BOLT SIZES AND LOCATIONS BY METAL BUILDING SUPPLIER.
   - B. SEE 'METAL BUILDING CRITERIA' ON SHEET S100.

FOOTING SCHEDULE			
MARK	SIZE (WxLxD)	TOP REINFORCING	BOT REINFORCING
F-1	9'-0"x9'-0"x1'-6"	(9) #7 BARS EA WAY	(9) #7 BARS EA WAY
F-2	5'-0"x5'-0"x1'-0"	(6) #5 BARS EA WAY	(6) #5 BARS EA WAY

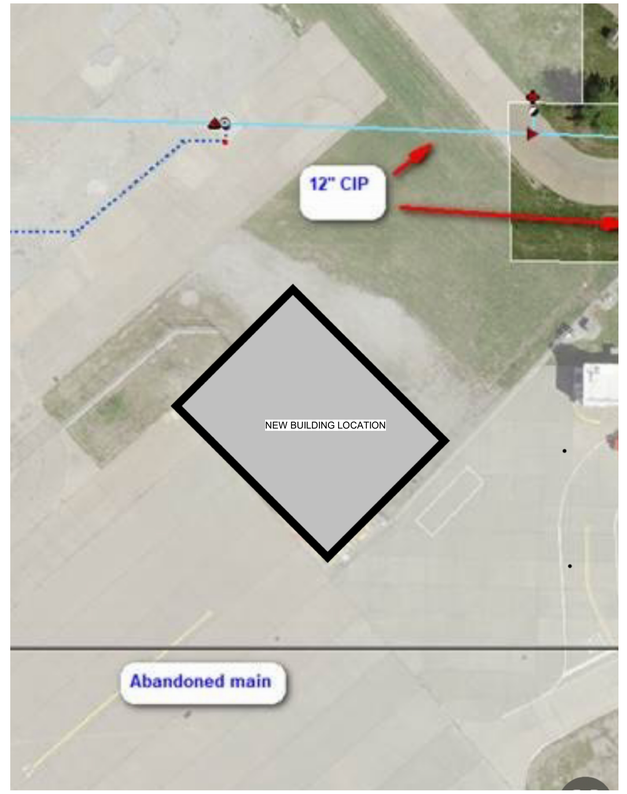
FOUNDATION WALL SCHEDULE		
WALL THICKNESS	REINFORCING	COMMENTS
8 IN.	#4 @ 12" O.C. EA WAY	CENTERED
12 IN.	#4 @ 12" O.C. EA WAY	EA FACE

KEYNOTES:

WALL RECESSED 8" FOR SLAB POUR OVER.

(HP) HAIRPIN - SEE DETAILS *2/S300* & *1/S301*.

THESE PLANS REQUIRE FINISHED GRADE AT THE BUILDING PERIMETER TO BE WITHIN 6 INCHES OF TOP OF INTERIOR BUILDING SLAB ELEVATION. NOTIFIY THE EOR IF FINAL GRADES DO NOT MEET THIS REQUIREMENT. SEE SITE EXHIBIT DRAWING BY CMT ENGINEERS AND CONSULTANTS DATED AUGUST 13, 2025 FOR EXACT BUILDING LOCATION.



BUILDING LOCATION AERIAL

NO SCALE

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FOUNDATION PLAN

S200

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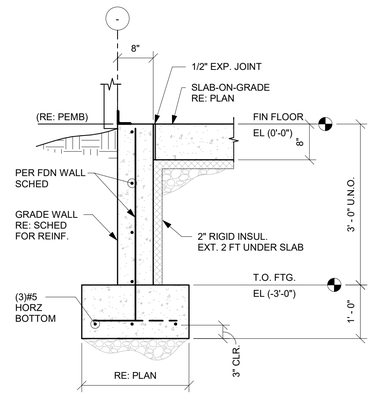
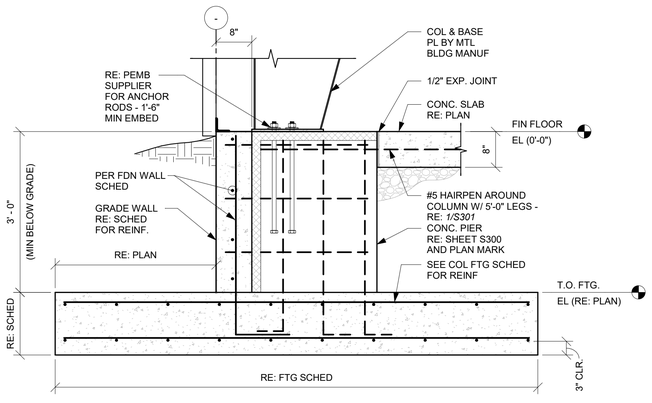


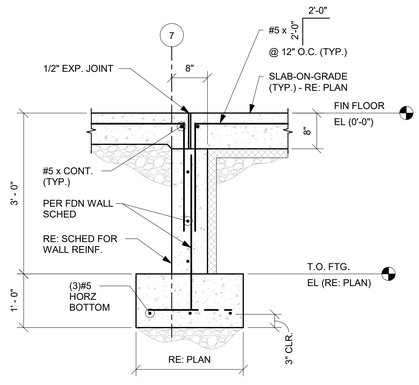
1 TYPICAL FOUNDATION WALL

3/4" = 1'-0"



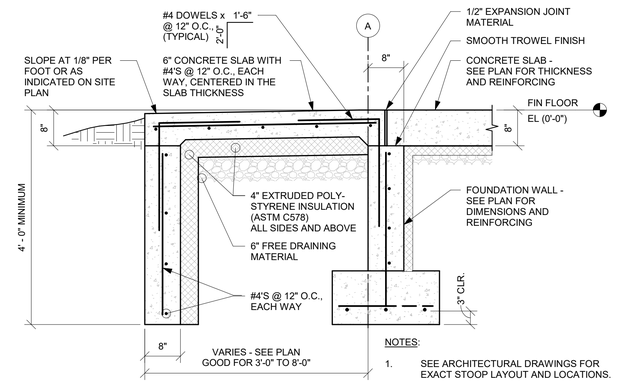
2 TYPICAL EXTERIOR COL FOOTING

3/4" = 1'-0"



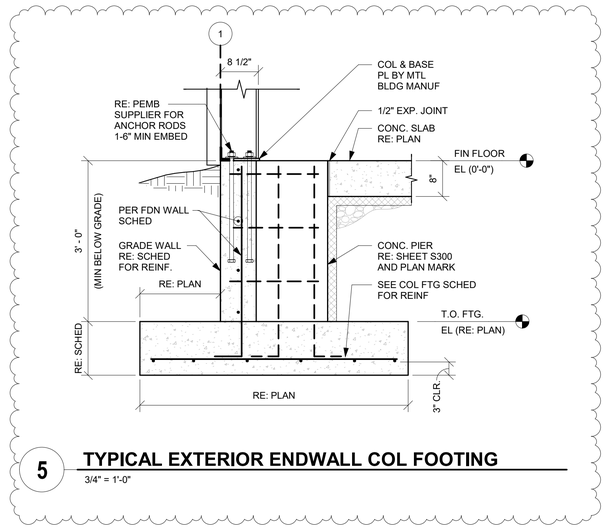
3 TYPICAL THRESHOLD FOUNDATION WALL

3/4" = 1'-0"



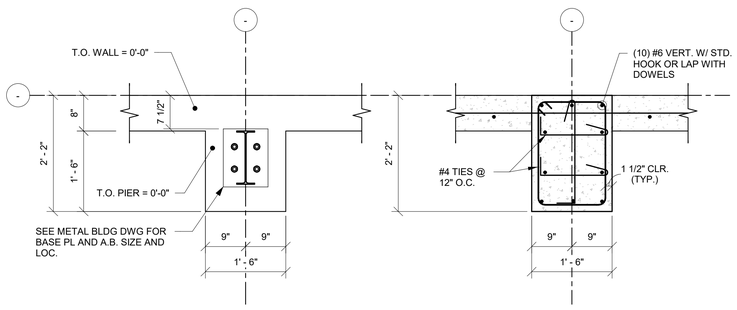
4 TYPICAL STOOP SECTION

3/4" = 1'-0"



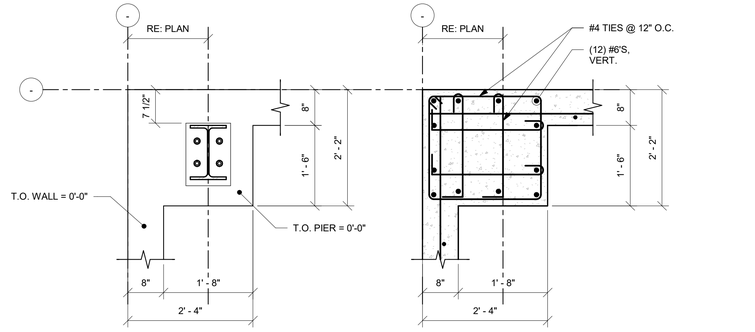
5 TYPICAL EXTERIOR ENDWALL COL FOOTING

3/4" = 1'-0"



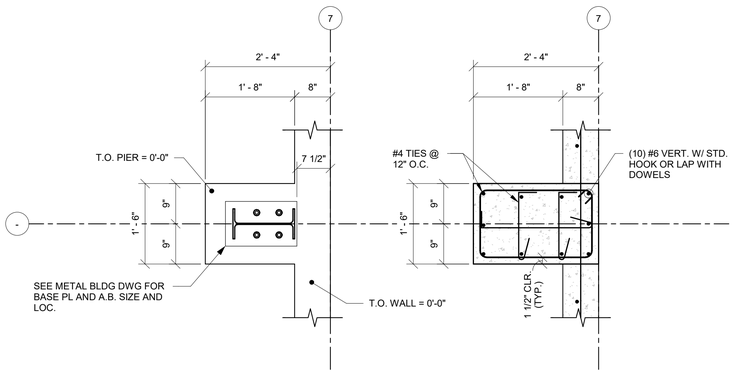
6 'P1' PIER DETAIL

3/4" = 1'-0"



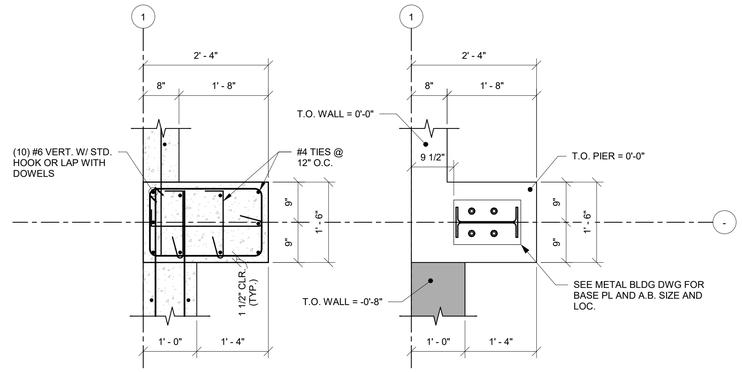
7 'P2' PIER DETAIL

3/4" = 1'-0"



9 'P3' PIER DETAIL

3/4" = 1'-0"



10 'P4' PIER DETAIL

3/4" = 1'-0"

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FOUNDATION DETAILS

S300

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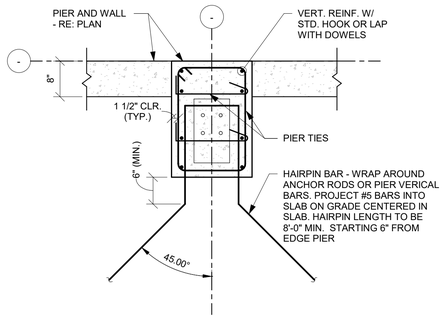
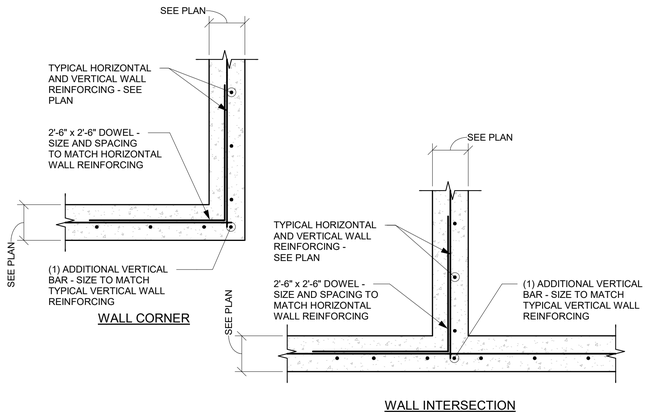


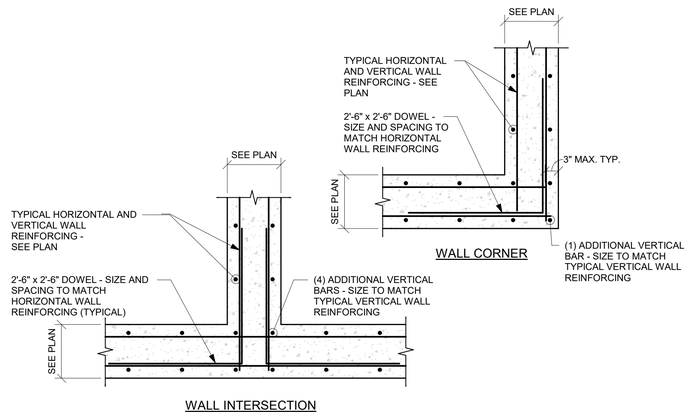
1 TYPICAL HAIRPIN DETAIL

3/4" = 1'-0"



2 TYPICAL CONCRETE WALL DETAILS

3/4" = 1'-0"



3 TYPICAL CONCRETE WALL DETAIL

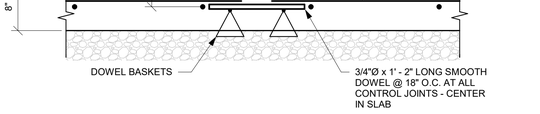
3/4" = 1'-0"

4 TYPICAL FOOTING CORNER BARS

1/2" = 1'-0"

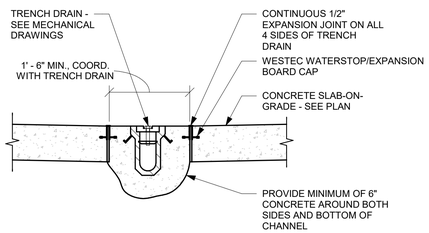
5 TYPICAL SLAB-ON-GRADE SECTION

1 1/2" = 1'-0"



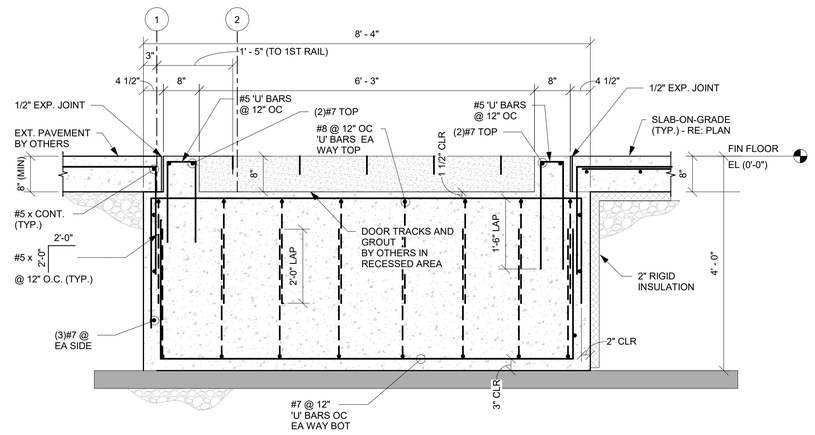
6 TYPICAL CONTROL OR CONSTRUCTION JOINT DETAIL

1" = 1'-0"



7 TRENCH DRAIN DETAIL

3/4" = 1'-0"



8 TYPICAL THRESHOLD FOUNDATION WALL

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S301

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